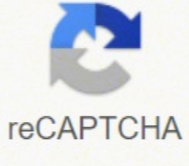




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Dalhousie University (Sexton Campus)  
Department of Civil & Resource Engineering

CIVL 4440 – Water and Wastewater Treatment  
Water Treatment Plant Design Project

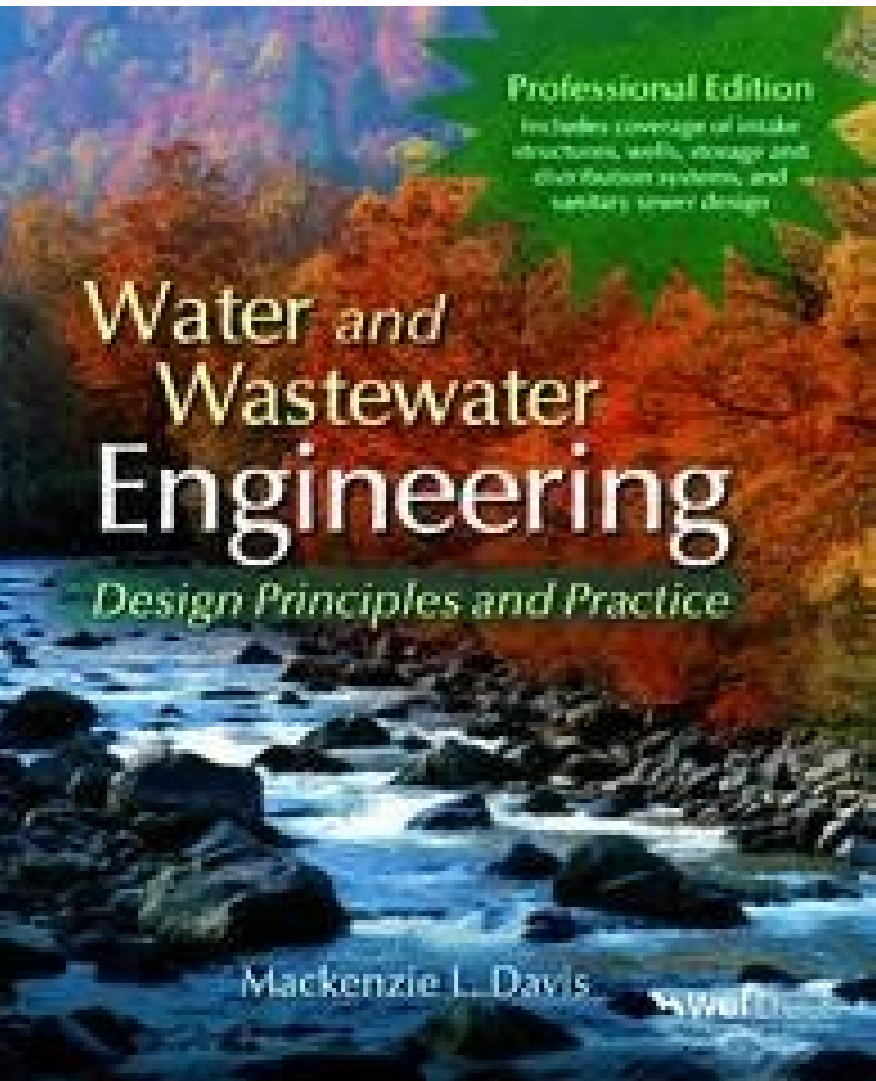
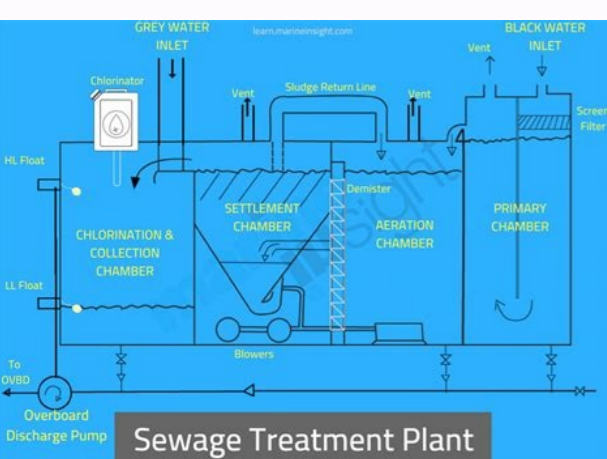
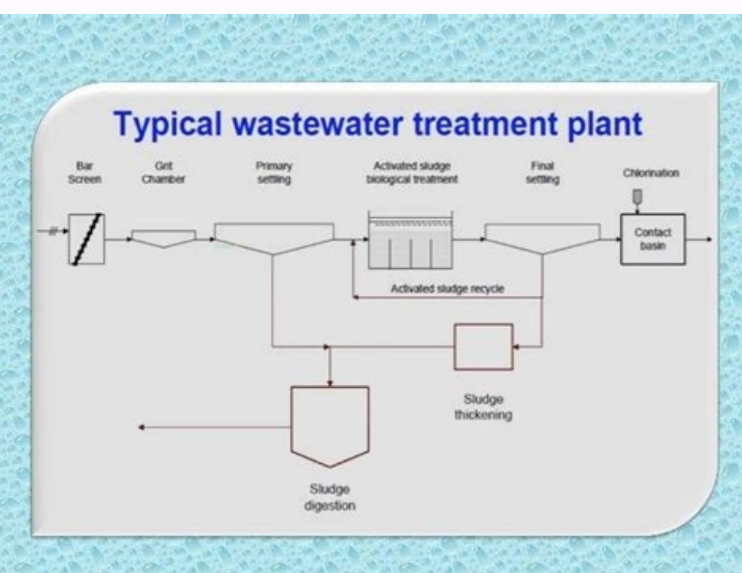
**GROUP #7**

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Submitted to:  
Dr. Margaret Walsh

Client:  
Town of Ferguson

December 9<sup>th</sup>, 2016



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Those results have done for final clarifier 1 and clarifier 2 for secondary treatment process. The pumping station or pump house at the treatment plant is normally of RCC and consists of a wet well and a dry well. It is a mixture of toilet water, washing water, ice, bathing, washing clothes, and all cleaning work at home, institutions, streets and rainwater. The results showed that the increasing of the number of loading cycles is presented by the decreasing of the ultimate lateral capacity. Thus, studying the properties of wastewater has to be investigated because all important in terms of plant layout, plant design, plant sizing and plant location. This system is commonly used in Iraq because it is known technology with less expensive sludge treatment requirement and it can satisfy the standards criteria for effluent disposal to surface water. The estimated flowrates for phase I and phase II are: Phase #1 (2030)  $Q_{ave} = 27,826.481159.437,396.481558.18$   $Q_{peak} = 62,442.62177,560.299$   $Q_{min} = 12,466.26317,576.345$  Table 7. Computation of wastewater discharge peak factors  
Peak factor equation:  $PF = 1 + \frac{A}{\sqrt{P}}$   
For population  $< 80,000$   $PF = 1.48 + \frac{0.78}{\sqrt{P}}$   
For population  $> 80,000$   $PF = 1.48 + \frac{0.78}{\sqrt{P}}$

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